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Measuring mode I cohesive law of wood bonded joints based on digital image correlation and fibre Bragg grating sensors



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ABSTRACT

This work addresses the experimental identification of mode I cohesive law of wood bonded joints. The approach combines the double cantilever beam (DCB) test with both digital image correlation (DIC) and embedded fibre Bragg grating (FBG) sensors. The spectrum geometric mean of the FBG reflected spectral response was determined, and the wavelength evolution was used to define the fracture process zone (FPZ) development phase. This evaluation allowed a consistent selection of experimental range of over which the identification procedure of mode I cohesive law is build up. Mode I crack length, Resistance-curve and cohesive law parameters are characterised and discussed. The strain energy release rate (G_1) is determined from the P- δ curve by the compliance-based beam method (CBBM). The crack tip opening displacement (w_1) is determined by post-processing displacements measured by DIC. The cohesive law in mode I (σ_1 - w_1) is then obtained by numerical differentiation of the G_1 - w_1 relationship.

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1. Introduction

Adhesive technologies have been playing an important role in new applications and innovation of modern wooden constructions. Although there have been great advances in the science and engineering of wood adhesion, the characterisation and modelling of the adhesive bond strength at the adherent wood interface remains an open problem. Moreover, the crack initiation and propagation in adhesives evolve into a fracture process zone (FPZ) ahead of the crack tip. An effective way to deal with such complex fracture phenomena and mechanisms is through cohesive zone models (CZM), conjointly with interface finite elements [1-4]. Accordingly, the entire FPZ is collapsed into the cohesive crack surfaces, being the material behaviour described by a cohesive law that relates the cohesive stresses to the relative displacements of the adjacent cohesive surfaces [3]. This approach has some advantages with regard to the classical fracture mechanics approach. A relevant one is the possibility to completely describe

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the fracture event, starting from the undamaged material, without the presence of an initial crack and it is not almost mesh independent.

Although the widespread use of CZM, the experimental identification of the cohesive laws poses several difficulties. One way of tackling this problem is by an inverse method, in which a particular shape for the cohesive law is assumed and its constitutive parameters determined from global experimental tests using optimisation procedures [5–7]. However, this type of indirect identification method is impaired with an important limitation. Indeed, the a priori choice of a shape for the cohesive law does not guarantee its transferability from the laboratory tests to an arbitrary geometry. Moreover, this approach can be computationally time consuming and the uniqueness of the minimisation problem is not guaranteed. In order to overcome these drawbacks, several authors [8-11] have developed direct methods to identify the cohesive law without any prerequisite assumption about its shape. They are based on the simultaneous measurement of the crack opening displacement (COD) and the related strain energy release rate (G) during the fracture test. The cohesive law comes up through the differentiation of G versus COD relationship. This approach was recently applied to wood and composites bonded joints under both mode I [6,12,13] and mode II [14,15] loading. The opening displacement at the initial crack tip (CTOD) was

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evaluated by post-processing the crack tip displacements measured by means of digital image correlation (DIC) [16,17], whereas Resistance-curves (R-curves) were determined using the compliance-based beam method (CBBM) [5]. However, the identification of the cohesive law must be obtained from the experimental response of the specimen during the development of the FPZ, i.e., after the initial elastic response of the specimen up to the self-similar crack propagation. Hence, a central issue in determining the cohesive law directly from load (P), load-point displacement (δ) and CTOD is the clear identification of the FPZ development phase. In the case of bonded joints, embedded fibre Bragg grating (FBG) sensors can be a feasible way to accomplish this goal [18].

A direct method for evaluating the cohesive laws of *Pinus pinaster* Ait. wood bonded joints under mode I is examined in the present work. The double cantilever beam (DCB) test was selected. The identification method requires the continuous monitoring of CTOD and the evaluation of the strain energy release rate in mode I (G_1). A simple data reduction scheme (CBBM) based on the elastic crack equivalent concept (a_e) is used to determine the evolution of G_1 during the test. DIC was employed for determine CTOD in mode I (w_1). An FBG sensor embedded in the glue line, at the initial crack tip, was employed in order to assist in the identification of FPZ development phase. These measurements were analysed in view of the direct identification of mode I cohesive law of wood bonded joints.

2. Mode I cohesive law

The DCB test, coupled with DIC and FGB measurements, is schematically shown in Fig. 1. The specimen consists of two $L \times h \times B$ rectangular beams, glued by an adhesive with thickness t. Moreover, an initial crack length a_0 is assumed in the reference configuration. The basic equation to evaluate $G_{\rm I}$ is the Irwin–Kies equation,

$$G_{I} = \frac{P^{2}}{2B} \frac{dC}{da} \tag{1}$$

where C and a are the specimen compliance and crack length in the current configuration, respectively. The CBBM is a straightforward data reduction scheme to determine $\mathrm{d}C/\mathrm{d}a$, circumventing the previous calibration of specimen compliance or the monitoring of crack

length during the fracture test [5,19]. Furthermore, the CBBM is suitable to deal with the variability of elastic properties, which is particularly important for biological materials. Considering the Timoshenko beam theory and the Castigliano theorem, it can be shown that *C* is given explicitly as a function of *a* by the following relationship [5]

$$C = \frac{8a^3}{Bh^3 E_L} + \frac{12a}{5BhG_{LR}} \tag{2}$$

where $E_{\rm L}$ and $G_{\rm LR}$ represent the elastic longitudinal and shear moduli, respectively. By replacing the initial crack length and specimen compliance (i.e., a_0 and C_0) in Eq. (2), an equivalent elastic modulus ($E_{\rm f}$) can be obtained that accounts for the presence of adhesive, material variability between specimens and stress concentration in the vicinity of the crack tip

$$E_{\rm f} = \frac{8(a_0 + h\Delta)^3}{Bh^3} \left(C_0 - \frac{12(a_0 + h\Delta)}{5BhG_{LR}} \right)^{-1} \eqno(3)$$

where Δ is a crack length correction that accounts for root rotation elastic effects. This parameter can be obtained numerically in two steps [20]. Firstly, the initial compliance is adjusted for the considered a_0 . Secondly, two numerical simulations considering two different initial crack lengths must be performed in order to establish the $C^{1/3} = f(a)$ relation. The interception of this line with the abscissa axis allows the evaluation of Δ . During the crack propagation, Eq. (2) can be used to determine the equivalent elastic crack length (a_e) from the current specimen compliance (C), taking into account $E_f(\text{Eq.}(3))$ instead of E_L . The R-curve in mode I can therefore be obtained combining the Irwin–Kies equation (1) with Eq. (2) yielding

$$G_{\rm I} = \frac{6P^2}{B^2h} \left(\frac{2a_{\rm e}^2}{E_{\rm f}h^2} + \frac{1}{5G_{\rm LR}} \right). \tag{4}$$

It should be referred that G_{LR} has a minor influence on the results [5] which means that a typical value can be used. Experimentally, the elastic properties of wood can be determined from both quasi-pure mechanical tests such as off-axis, Iosipescu and Arcan [21,22] or heterogeneous tests such as the unnotched Iosipescu test [23,24] or annual ring-pattern tests [25].

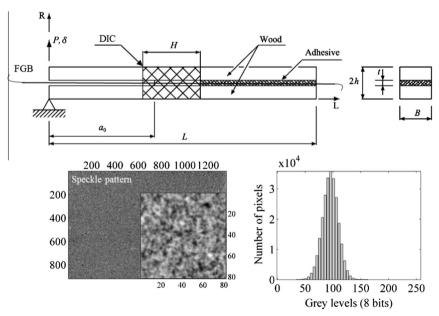


Fig. 1. Schema of the DCB test (2h = 20 mm, B = 20 mm, t = 0.1 mm, L = 500 mm, H = 40 mm, and $a_0 = 100 \text{ mm}$) coupled with DIC (speckle pattern and its histogram over a region of interest of $24.5 \times 16.6 \text{ mm}^2$) and FBG.

The direct identification of the cohesive law in mode I requires the measurement of CTOD, with a suitable technique such as DIC. The cohesive law arises from the differentiation of G_I with respect to the CTOD (w_I)

$$\sigma_{\rm I}(w_{\rm I}) = \frac{{\rm d}G_{\rm I}}{{\rm d}w_{\rm I}} \tag{5}$$

A suitable differentiation algorithm must be used to avoid noise amplification in the reconstruction of the constitutive cohesive law. In order to apply Eq. (5), it is proposed to fit the $G_{\rm I}-w_{\rm I}$ data by a continuous function described by the following expression (logistic function)

$$G_{\rm I} = \frac{A_1 - A_2}{1 + (w_{\rm I}/w_{\rm I0})^p} + A_2 \tag{6}$$

where A_1 , A_2 , p and $w_{II,0}$ are constants to be determined by non-linear least-square regression [26]. In this function, the A_2 parameter must provide an estimation of the critical strain energy release rate in the sense that: $A_2 \equiv G_{\rm Ic}$ when $w_{\rm I} \rightarrow \infty$.

3. Experimental work

3.1. Material and testing

The wood material used in this work was taken from a single *Pinus pinaster* Ait. tree. Matched beams were cut from mature wood to mitigate material variability. DCB wood-bonded joints specimens were then prepared with nominal dimensions of 2h = 20 mm, B = 20 mm, t = 0.1 mm, L = 500 mm and an initial crack length of $a_0 = 100$ mm (Fig. 1). The wood surfaces were firstly clean and polished using sandpaper to enhance adhesion with the epoxy adhesive (Araldite® 2015). The pre-crack surface within the adhesive was introduced by means of a thin Teflon® film (thickness of 25 um).

The DCB fracture tests were carried out in a screw-driven Instron® 1125 testing machine at crosshead displacement rate of 5 mm/min. The applied load was measured by means of a 100 kN load cell. The load (P) and the crosshead displacement of the testing machine (δ) were recorded during the test by means of a National Instruments® USB-6210 daq card controlled using Labview®, with a sampling rate of 10 Hz, connected to the analogue outputs of the testing machine. In order to define the $G_{\rm I}$ = f(w) function, the displacement fields at the crack tip during the fracture tests were measured by DIC as described in the following.

3.2. Digital image correlation

DIC is a full-field optical technique in which displacements at discrete data points are provided by correlating images at different mechanical configurations [27]. The deformation of the material is evaluated by means of the deformation of a textured pattern, assumed perfectly attached to the object surface. This technique has been developing in recent decades and is gradually becoming a general-proposed tool in experimental mechanics. The spatial resolution and accuracy of the technique can be adjusted by the optical components used in the image formation and size and step of correlation windows, in order to address engineering applications that can be scaled from structural to micro levels.

At the macro scale, the natural wood surface does not provide the required textured pattern for DIC measurements. A speckle pattern was then painted over a region of interest of $40(R) \times 20(T) \, \text{mm}^2$, around and ahead of the initial crack tip, by means of an airbrush, to guarantee suitable grain size, contrast and isotropy at the scale of observation (Fig. 1). The ARAMIS® DIC-2D system was used in this work [28,29]. A charged coupled

digital (CCD) camera coupled with a telecentric lens was used for image grabbing. The optical system components and DIC measuring parameters are summarised in Table 1. According to the typical size of the painted speckle pattern, a subset size of 15×15 pixels² $(0.27 \times 0.27 \text{ mm}^2)$ was chosen. This length defines the spatial resolution associated to the measurements (i.e., the smallest distance separating two independent measurements). The accuracy of the measurements was estimated experimentally based on translation rigid-body tests, from which displacement and strain fields are theoretically known [29,30]. At least a pair of images was recorded before and after applying a lateral translating of the camera-lens system using a X-Y translation table. Typically, noise fields are obtained from image correlation, which can be modelled as a white Gaussian noise signal. Therefore, the standard deviation was defined as an estimation of the global accuracy associated to the optical measurements. A resolution in displacement of the order of $1-2 \times 10^{-2}$ pixel (0.18–0.36 µm) was achieved.

The value of w_1 must be determined locally at the initial crack tip with suitable spatial resolution and accuracy. A solution can be provided by post-processing DIC displacements according to the following relationship

$$w_{\rm I} = \|w_{\rm I}^+ - w_{\rm I}^-\| \tag{7}$$

where $w_{\rm l}^+$ and $w_{\rm l}^-$ are the components of the displacement in the direction perpendicular to the crack propagation, associated to the upper and the lower cracked surface. Numerically, this parameter can be determined at superimposed nodes located at the initial crack tip of cohesive elements with zero thickness. Experimentally, this relative displacement was evaluated at a pair of subsets, chosen up and down the crack tip location, with a spatial resolution of 0.448 mm.

3.3. Fibre Bragg grating sensors

Fibre optic FBGs have been used as a strain sensor in many engineering applications. One intrinsic advantage is related to the sensor dimension due to the fibre diameter (125 µm for a standard SMF28 fibre). For the purpose of this study, the FBG was conveniently embedded in the epoxy adhesive within the wooden joint. FBGs with a length of 6 mm were produced by the phase mask technique using a KrF excimer laser emitting 24 ns pulses with wavelength of 248 nm. These sensors were manufactured by Fibersensing®, with the Bragg peak at 1550 nm and a peak reflectivity higher than 90%. The fibre with FBG was embedded in the glue line, during the specimen manufacture, ensuring that the sensor was positioned at the centre of initial crack tip and oriented along the longitudinal direction of specimen (Fig. 2). The correct alignment of optical fibre was ensured using dead weights attached to its extremities, while pressing the two arms of DCB specimen during the adhesive cure. The fibre was connected to a spectral analyser Fibersensing® Braggmeter FS2200SA that retrieved reflection spectra with a frequency of 0.5 Hz. This system makes the FBG interrogation in the 1500-1600 nm range and can record simultaneously the reflection and transmission spectra. Both the spectral analyzer and the daq card were controlled by the same computer and all the data acquired by these two systems were temporally synchronised.

4. Results and discussion

Fig. 3 shows a P- δ curve obtained experimentally, which is representative of the typical behaviour observed. After the initial elastic response, the curve shows a non-linear behaviour before the maximum load is reached, which is attributed to the formation and growth of the FPZ ahead of the initial crack tip [31]. After this stage, a monotonically decreasing of the load is observed

Table 1Components of the optical system and digital image correlation measuring parameters.

CCD camera	
Model	Baumer Optronic FWX20
	(8 bits, 1624×1236 pixels, $4.4 \mu m/pixel$)
Shutter time	0.7 ms
Acquisition frequency	1 Hz
Lens	
Model	Opto Engineering Telecentric lens TC 23 36
Magnification	0.243 ± 3%
Field of view (1/1.8")	$29.3 \times 22.1 \text{ mm}^2$
Working distance	103.5 ± 3 mm
Working F-number	f/8
Field depth	11 mm
Conversion factor	0.018 mm/pixel
Lighting	Raylux 25 white-light LED
DIC measurements	
Subset size	$15 \times 15 \text{ pixel}^2 (0.270 \times 0.270 \text{ mm}^2)$
Subset step	$13 \times 13 \text{ pixel}^2 (0.234 \times 0.234 \text{ mm}^2)$
Resolution	$1-2 \times 10^{-2}$ pixel (0.18–0.36 µm)

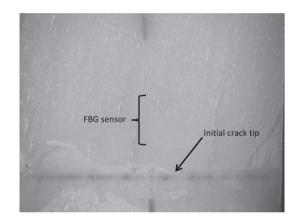


Fig. 2. FBG sensor location and typical failure of the wood bonded joints.

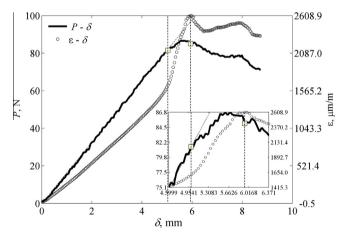


Fig. 3. Load-displacement and mean local strain-displacement curves.

corresponding to steady-state crack propagation. Fig. 4 shows the evolution of CTOD with regards to the applied displacement (δ) (for the same specimen of Fig. 3). In the CTOD- δ curve, three distinct regions can be distinguished in close relation with the P- δ one. In the first region, the CTOD increases at an almost constant rate, corresponding to the initial elastic response in the P- δ curve. The second region is characterised by a CTOD growth at increasing rate, and can be linked to the non-linear section of the P- δ curve

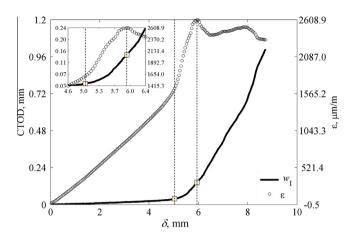


Fig. 4. CTOD-displacement and mean local strain-displacement curves.

before maximum load is reached. Hence, the behaviour of this second region of the CTOD- δ curve is conceivably a consequence of the development of the FPZ. The last region of CTOD- δ curve exhibits again a constant CTOD growth rate and is related to the monotonically decreasing region of P- δ curve associated to the steady-state crack propagation. The failure of all specimens takes place in the adhesive, as is shown in the photograph of Fig. 2.

The nature and sequence of damage events described above, inferred from the experimental $P-\delta$ and $CTOD-\delta$ curves, was also examined using the reflected spectral response of the FBG sensor embedded in the glue line at the initial crack tip. The typical FBG reflection spectrum evolution is shown in Fig. 5, in which the spectrum wavelength shift is plotted at three different applied loads. The change in the shape is a result of the high heterogeneous strain applied to the fibre in the FBG position by the glued joint. Just after the maximum applied load, the reflection spectrum returns to the original shape corresponding to the situation where the glued joint has collapsed and the crack propagated beyond the FBG position.

The reflection spectra are difficult to interpret due to peak splitting and the appearance of multiple reflection peaks of equivalent height [32]. Several methods have been proposed to reconstruct the multiaxial strain field applied to the FBG. However, these techniques are, in general, very time consuming and provide limited information of the specimen overall mechanical behaviour [33]. To determine the spectral shift the standard methods try to identify one maximum reflection peak per FBG and follow the position of this peak in time [34]. However, if the reflected spectrum is becoming highly deformed with multiple peaks splitting, each

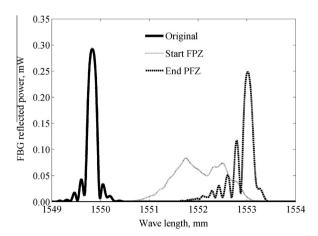


Fig. 5. Spectral responses at three stages of loading: at the beginning of test (original spectrum), at the onset of FPZ formation and at the end of full extension of FPZ

FBG will yield multiple peaks with equivalent heights resulting in inconsistent values for the spectrum wavelength shift evolution. It was also observed that the overall FBG spectral reflection response always moves toward higher wavelengths with the increase of applied deformation even if it was not possible to identify a predominant reflection peak that can be followed and used to quantify this overall wavelength shift. This problem was tackled by the use of an alternative method to quantify the FBG spectral wavelength shift using the equivalent to a spectral wavelength geometric mean (λ_{CM} that can be determined for all the measured spectra. The evolution of this wavelength geometric mean can be followed in time and then related with the other macroscopic quantities measured simultaneously during the DCB test.

In this method, each spectrum is divided in m optical power levels, between 10% and 95% of maximum FBG reflected power (p_{max}). The geometric mean of each power level (p_i) is determined by

$$\lambda_j^{\text{avg}} = \sqrt[n]{\prod_{i=1}^n \lambda_i} \tag{8}$$

using the n wavelengths (λ_i) that satisfy the inequality: $p_i \geqslant p_j$. The number of terms satisfying this condition will vary among power levels and, as a consequence, the root order will also vary accordingly. The final wavelength representing each spectrum is then determined by calculating the geometric mean of the wavelengths representing the m power levels by

$$\lambda_{\rm GM} = \sqrt[m]{\prod_{i=1}^{m} \lambda_i^{\rm avg}} \tag{9}$$

By using this method the power distribution is taken into account because wavelengths with higher power levels will contribute more to the final mean value than wavelengths with lower power levels. Fig. 6 shows the evolution of spectral geometric mean for the different power levels and the final value for each of the two spectra presented. It should be noted that in non-deformed and symmetric FBG reflection spectra, $\lambda_{\rm GM}$ is identical to the measured by the standard methods of peak identification and determination, as it can be observed in the initial FBG spectra in Fig. 6, in particular when compared to the values returned by Braggmeter interrogation system.

The mean wavelength shift $(\Delta \lambda_{GM})$ is related to the mean local strain at the crack tip (ε) by the following equation [35]:

$$\varepsilon = \frac{1}{1 - p_e} \frac{\Delta \lambda_{\text{GM}}}{\lambda_{\text{GM0}}} \tag{10}$$

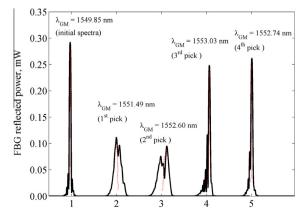


Fig. 6. Reflection spectra of an FBG with wavelength geometric mean for different power levels (red line inside the spectra) and the geometric mean of all power levels considered. (For interpretation of the references to color in this figure legend, the reader is referred to the web version of this article.)

where p_e = 0.22 is the photo-elastic coefficient for standard Gedoped optical fibres and λ_{GM0} is the initial (before the fracture test) value of λ_{GM} .

The variation of the mean local strain (ε) with regard to the applied displacement (δ) is shown in Figs. 3 and 4, together with the $P-\delta$ curve and the CTOD- δ curve, respectively. A noticeable rise in the local strain rate is observed, in close agreement with the increase of CTOD growth rate (Fig. 4), after the end of the initial elastic segment of $P-\delta$ curve (Fig. 3). That change of local strain rate is understood as an indicator of the onset of FPZ development. Hence, the following criterion for the initiation of FPZ can be stated from the $\varepsilon-\delta$ curve: it is the displacement δ_i obtained from the intersection of two straight lines adjusted to the $\varepsilon-\delta$ data points, before and after the main slope augmentation of $\varepsilon-\delta$ curve. This criterion is translated to a point (square point) in the $P-\delta$ curve (Fig. 3) and in the CTOD- δ curve (Fig. 4), showing its consistency.

As it can be seen in Fig. 3, in the neighbourhood of the maximum load applied to the specimen (for this specimen, the square point slightly after the maximum load) the ε - δ curve attains a maximum value, which is followed by a negative slope. This negative slope represents a strain relaxation which could be attributed to the start of crack advance and to the end of FPZ. The displacement δ_e matching the maximum ε value (or the first sign change of ε - δ slope) can be employed as a criterion to define the full development of FPZ. The CTOD value associated to δ_e is represented by a square point in Fig. 4. As it can be seen, this point is the boundary between the second and third regions of CTOD- δ curve discussed above.

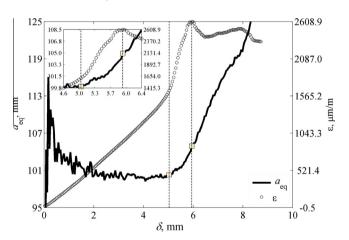


Fig. 7. Equivalent crack length versus displacement and mean local strain versus displacement curve.

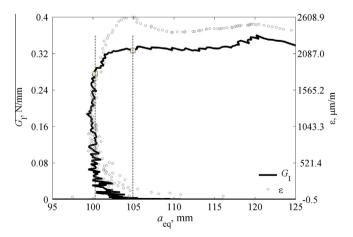


Fig. 8. Mode I resistance-curve and mean local strain versus equivalent crack length curve.

The wavelength peak (or Bragg wavelength) is a very sensitive parameter and its value can be measured accurately when the applied strain is uniform along the grating, as shown in the original spectrum of Fig. 5. However, during the fracture test the spectral response becomes very complex due to the presence of nonuniform axial strains, displaying split-peak or multiple-peaks, with no simple means of interpretation. Moreover, the form of FBG reflection spectrum changes considerably during the fracture test, according to the local damage events. Fig. 4 shows the original spectrum (at the beginning of fracture test) along with the spectra at the limits of FPZ development, identified before (Figs. 3 and 4). Despite the complex form and evolution of FBG reflection spectrum throughout the fracture test, the geometric mean wavelength λ_{GM} (or the related local mean strain, obtained through Eq. (10)) proves to be an effective way to determine the limits, in terms of the independent testing variable (i.e. the applied displacement). of FPZ development. Therefore, the identification of mode I cohesive law should be performed using the P, δ and CTOD experimental values within those limits.

The first step in applying the CBBM is the evaluation of the equivalent elastic crack length $(a_{\rm e})$ from the $P-\delta$ curve, using Eqs. (2) and (3). Fig. 7 shows the variation of $a_{\rm e}$ with the applied displacement, together with the $\varepsilon-\delta$ curve. The first significant value of $a_{\rm e}$ occurs at the onset of FPZ (first square mark in Fig. 7) and is equal to the initial crack length (a_0) . In the early stages of linear elastic response of specimens the calculated values of $a_{\rm e}$ are noisily and its evolution with the applied displacement is meaningless. It should be noticed that $a_{\rm e}$ was calculated from the current compliance (Eq. (2)), on a point-by-point basis. After the onset of FPZ, $a_{\rm e}$

Table 2 Evaluation of the equivalent crack length during FPZ development phase (Δa_e) and initial (i) critical (c) and at maximum load (P_{max}) strain energy release rate (G_1) of P. *pinaster* wood bonded joints.

Specimen	$\Delta a_{\rm eq} ({\rm mm})$	G _{li} (N/mm)	G _{Ic} (N/mm)	$G_{I,Pmax}$ (N/mm)
1	3.149	0.335	0.430	0.435
2	13.1	0.289	0.514	0.464
3	5.134	0.252	0.326	0.318
4	9.022	0.265	0.484	0.508
5	3.843	0.211	0.412	0.460
6	4.734	0.111	0.224	0.224
7	3.144	0.252	0.371	0.368
8	4.695	0.148	0.253	0.253
9	4.740	0.214	0.309	0.312
10	13.7	0.210	0.448	0.369
11	4.625	0.183	0.348	0.348
Mean	6.347	0.225	0.374	0.364
Std	3.807	0.064	0.093	0.088
C.V. ¹ (%)	60.0	28.3	24.8	24.1

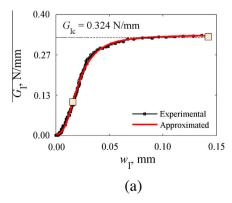
¹ Coefficient of variation (C.V.).

advances at a growing rate, until a constant propagation rate is achieved. It is remarkably noted that the transient region of equivalent crack propagation (i.e., the region where da_e/dt is growing) matches with the FPZ development identified in the ε - δ curve. This behaviour was systematically observed among the tested specimens. The second step of the data reduction is the evaluation of the R-curve by applying Eq. (4) to (δ, P) experimental data. The obtained results can be seen in Fig. 8, where the λ_{GM} versus a_e relationship is also superimposed. An initial (G_{li}) and critical (G_{lc}) strain energy release rates in mode I were evaluated from the R-curves afterwards. G_{li} is assumed to be the value of strain energy release rate at δ_i (i.e., at the onset of FPZ: first square mark in Fig. 8), whereas G_{lc} is assumed to be the strain energy release rate at δ_e , just before steady-state crack propagation (i.e., at the end of FPZ development: second square mark in Fig. 8). It is worth noticing that the ε - a_e curve exhibits the same trend of the R-curve, in the pre-FPZ development region and in the post-FPZ development

Table 2 summarises the results of the equivalent crack length during FPZ development phase ($\Delta a_{\rm e}$), initial ($G_{\rm Ii}$) and critical ($G_{\rm Ic}$) strain energy release rates, extracted from the *R*-curve at the boundaries of the FPZ development as described above. Table 2 also contains the results of strain energy release rate for the maximum load ($G_{\rm I,Pmax}$), for comparison purposes. An average value of $\Delta a_{\rm e}$ = 6.347 mm is obtained for the FPZ extension. In terms of $G_{\rm I,t}$ taking into account the scatter of those results, the mean value of $G_{\rm I,Pmax}$ can be taken as a practical measure of the mode I critical strain energy release rate.

Fig. 9a shows the $G_I = f(w_I)$ curve obtained by combining CBBM and DIC measurements. This curve is the basic information required for the direct identification of the cohesive law (Eq. (5)). A continuous function (in this work, the logistic function, Eq. (6)) was used to approximate the data points in the reconstructing process of the cohesive law, as shown in Fig. 9a. This procedure allows filtering the experimental data before analytical differentiation, thus circumventing the noise amplification characteristic of numerical differentiation. Only the data points within the limits of FPZ development were taken into account in the regression analysis. The cohesive laws were then determined by analytical differentiation of the logistic approximation function, as shown in Fig. 9b.

Table 3 summarises the following parameters of cohesive law: the area under the cohesive law (which is a measure of critical strain energy release rate, $G_{\rm l,area}$), the maximum cohesive stress ($\sigma_{\rm u}$) and the maximum or critical CTOD ($w_{\rm lc}$). The mean value of $G_{\rm l,area}$ (Table 3) is 1.3% lower than the mean value of $G_{\rm lc}$ (Table 2), which was determined independently from R-curves at the end of FPZ development (second square mark in Fig. 8). However, considering the scatter, this difference is not statistically significant. This result gives confidence to the identification method of cohesive law presented in this work.



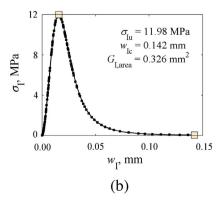


Fig. 9. (left) G_1 versus w_1 relation and logistic approximation function; (right) cohesive law $(\sigma_1 - w_1)$.

Table 3 Mode I cohesive law parameters of *P. pinaster* wood bonded joints.

Specimen	G _{I,area} (N/mm)	$\sigma_{\mathrm{u}}\left(\mathrm{MPa}\right)$	w _{Ic} (mm)
1	0.399	7.66	0.20
2	0.495	10.24	0.31
3	0.484	10.85	0.15
4	0.484	7.28	0.23
5	0.386	4.47	0.29
6	0.217	5.20	0.18
7	0.373	9.43	0.13
8	0.252	7.10	0.11
9	0.289	5.67	0.41
10	0.342	9.59	0.28
11	0.337	4.61	0.23
Mean	0.369	7.464	0.229
Std	0.094	2.307	0.089
C.V. ¹ (%)	25.5	30.9	38.9

¹ Coefficient of variation (C.V.).

5. Conclusions

This work addresses the experimental identification of the cohesive law in mode I of P. pinaster wood bonded joints. The approach combines the double cantilever beam (DCB) test with digital image correlation (DIC) and fibre Bragg grating (FBG) sensors embedded in the glue line. The strain energy release rate in mode I (G_I) is determined from the load–displacement (P– δ) curve by means of the compliance-based beam method (CBBM). This method relies on the concept of equivalent elastic crack length (a_e) , avoiding crack length monitoring during test. The crack tip opening displacement in mode I (w_I) was determined by postprocessing local displacements measured by DIC. The cohesive law in mode I $(\sigma_I - w_I)$ is then obtained by numerical differentiation of the $G_I - W_I$ relationship. DIC proved to be an efficient method for the experimental evaluation of the cohesive law. The geometric mean value of the wavelength shift of FBG reflected spectral response was used to detect the development of fracture process zone (FPZ), thus allowing the proper selection of experimental range of load (P), load-point displacement (δ) and CTOD values, over which the identification procedure of cohesive law is build up. The geometric mean value of the wavelength evolution of FBG reflected spectrum proves to be a valuable indicator in delimiting the FPZ development phase. An extension of about 6 mm of the FPZ on wood bonded joints was estimated. Moreover, it was found that the strain energy release rate at maximum load is a good estimation of G_{Ic} .

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